

8/31/99

Notes for beam bending problem 9/2

1. Set up the cantilever beam as beam theory

Define the load on the upper surface and write in terms of total force, F, figured positive if pointing up

```
STUDENT > q:=p*w*(x-L):  
           int(q,x=0..L):  
           p:=solve("=F,p):  
           q;
```

$$-2 \frac{F(x-L)}{L^2}$$

Integrate to get shear flow and moment

```
STUDENT > V:=V0-int(q,x):  
           Mb:=M0-int(V,x):
```

Set shear flow and moment equal to zero at the right hand end and solve for V0 and M0

```
STUDENT > x:=L:V0:=solve(V,V0):M0:=solve(Mb,M0):x:='x':
```

```
STUDENT > V0,M0;
```

$$F, \frac{1}{3}FL$$

And factor the shear flow and moment for convenience

```
STUDENT > V:=factor(V);Mb:=factor(Mb);
```

$$V := \frac{F(L-x)^2}{L^2}$$
$$Mb := \frac{1}{3} \frac{F(L-x)^3}{L^2}$$

2. Put in the approximate stresses that give the resultant shear flow and moments, working from the class notes

(Solve for the principal strains in terms of the stresses first.)

```
STUDENT > e1:=sx-2*mu*ex-lambda*(ex+ey+ez):  
           e2:=sy-2*mu*ey-lambda*(ex+ey+ez):  
           e3:=sz-2*mu*ez-lambda*(ex+ey+ez):  
           solve({e1,e2,e3},{ex,ey,ez}):  
STUDENT > assign("):  
           ex:=collect(ex,[sx,sy,sz]):  
           ey:=collect(ey,[sx,sy,sz]):  
           ez:=collect(ez,[sx,sy,sz]):  
           exs:=0:ey:=0:ezs:=0:
```

```

for i from 1 to 3 do
exs:=exs+factor(op(i,ex)):
eys:=eys+factor(op(i,ey)):
ezs:=ezs+factor(op(i,ez)):od:
exs;eys;ezs;

```

$$\begin{aligned}
& \frac{(\mu + \lambda) sx}{\mu (2\mu + 3\lambda)} - \frac{1}{2} \frac{\lambda sy}{\mu (2\mu + 3\lambda)} - \frac{1}{2} \frac{sz \lambda}{\mu (2\mu + 3\lambda)} \\
& - \frac{1}{2} \frac{\lambda sx}{\mu (2\mu + 3\lambda)} + \frac{(\mu + \lambda) sy}{\mu (2\mu + 3\lambda)} - \frac{1}{2} \frac{sz \lambda}{\mu (2\mu + 3\lambda)} \\
& - \frac{1}{2} \frac{\lambda sx}{\mu (2\mu + 3\lambda)} - \frac{1}{2} \frac{\lambda sy}{\mu (2\mu + 3\lambda)} + \frac{(\mu + \lambda) sz}{\mu (2\mu + 3\lambda)}
\end{aligned}$$

```

STUDENT > ex:=exs;ey:=eys;ez:=ezs;

```

Now I can plug in the model stresses from the class notes

```

STUDENT > sx:=-12*Mb*y/w/h^3;
sy:=12*q*(1/8*h^2*y-1/6*y^3+1/24*h^3)/w/h^3;
sz:=0;

```

$$\begin{aligned}
sx & := -4 \frac{F(L-x)^3 y}{L^2 w h^3} \\
sy & := -24 \frac{F(x-L) \left(\frac{1}{8} h^2 y - \frac{1}{6} y^3 + \frac{1}{24} h^3 \right)}{L^2 w h^3} \\
sz & := 0
\end{aligned}$$

And work on the strains

```

STUDENT > collect(ex,y);

```

ex = du/dx, so I can integrate this, remembering that u(0) = 0.

```

STUDENT > int(ex,x=0..x1):u:=subs(x1=x,");

```

```

STUDENT > factor(op(1,u))+factor(op(2,u));

```

$$\begin{aligned}
& \frac{1}{4} F(4y\mu L^4 - 16y\mu L^3 x + 24y\mu L^2 x^2 - 16y\mu L x^3 + 4y\mu x^4 + 4y L^4 - 16y L^3 x \\
& + 24y L^2 x^2 - 16y L x^3 + 4y x^4 + 3 x^2 h^2 y - 6 x h^2 y L - 4 x^2 y^3 + 8 x y^3 L + x^2 h^3 \\
& - 2 x h^3 L) / (L^2 w h^3 \mu (2\mu + 3)) - \frac{(\mu + \lambda) F L^2 y}{w h^3 \mu (2\mu + 3)}
\end{aligned}$$

```

STUDENT > u:="";

```

Similarly, I can get v by integrating ey, but I need to add in the displacement of the neutral axis, v0(x)

```

STUDENT > ey;

```

```

STUDENT > int(ey,y=0..y1):v:=subs(y1=y,")+v0(x);

```

```

STUDENT > factor(op(1,")+factor(op(2,"));

```

$$\frac{1}{2} F y (L - x) \frac{(2 y L^2 - 4 y x L + 3 \mu h^2 y - 2 \mu y^3 + 2 \mu h^3 + 3 h^2 y - 2 y^3 + 2 h^3 + 2 y x^2)}{(\mu + 3) L^2 w h^3} + v_0(x)$$

STUDENT > `v:=`:

Now the shear is given by $\tau = \mu \frac{dv}{dx}$, so I can calculate this from the candidate displacements

STUDENT > `gxy:=diff(u,y)+diff(v,x);`

$$g_{xy} := \frac{1}{4} F (4 \mu L^4 - 16 \mu L^3 x + 24 \mu L^2 x^2 - 16 \mu L x^3 + 4 \mu x^4 + 4 L^4 - 16 L^3 x + 24 L^2 x^2 - 16 L x^3 + 4 x^4 + 3 x^2 h^2 - 6 x h^2 L - 12 x^2 y^2 + 24 x y^2 L) / (L^2 w h^3 \mu (2 \mu + 3)) - \frac{(\mu + 3) F L^2}{w h^3 \mu (2 \mu + 3)} - \frac{1}{2} \frac{F y (2 y L^2 - 4 y x L + 3 \mu h^2 y - 2 \mu y^3 + 2 \mu h^3 + 3 h^2 y - 2 y^3 + 2 h^3 + 2 y x^2)}{\mu (2 \mu + 3) L^2 w h^3} + \frac{1}{2} \frac{F y (L - x) (-4 y L + 4 y x)}{\mu (2 \mu + 3) L^2 w h^3} + \frac{v_0(x)}{x}$$

The shear stress is known from our guess from beam theory

STUDENT > `txy:=6*V/w/h^3*((h/2)^2-y^2);`

$$t_{xy} := 6 \frac{F (L - x)^2 \frac{1}{4} h^2 - y^2}{L^2 w h^3}$$

and we take the difference between the shear stress divided by μ and the shear as calculated. (This would be zero were the solution from beam theory exact).

STUDENT > `txy/mu-gxy;`

We need to find dv_0/dx to make this zero (if that's possible)

STUDENT > `solve(",diff(v0(x),x)):`

STUDENT > `temp:=collect(",y):`

STUDENT > `test:=0:`

```
for i from 1 to nops(temp) do
test:=test+factor(op(i,temp)) od:
dv0dx:=test;
```

We know that h and y are of the same order, so let's gather the system up in terms of them

STUDENT > `collect(",[y,h],distributed);`

We can write y in terms of a dimensionless parameter η ($-1 \leq \eta \leq 1$) and h

STUDENT > `y:=eta*h/2:collect(dv0dx,h);`

And we see there are term that go like $1/h^3$, $1/h$ and h . h has dimensions. so that's not really quite enough to allow me to say that one term is bigger than another. h will be small compared to L , however, and we can write

```
STUDENT > h:=epsilon*L:dv0dx:=collect(dv0dx,epsilon);
```

And the we can also scale L , introducing ξ ($0 \leq \xi \leq 1$)

```
STUDENT > x:=L*xi:"":
```

So here we have the nice scaled version of $dv0dx$

```
STUDENT > factor(op(1,dv0dx))+factor(op(2,dv0dx))+factor(op(3,dv0dx));
```

$$-\frac{1}{16} \frac{F(\mu + \lambda) (-6 - 8 + \lambda^3)}{L w \mu (2\mu + 3)} - \frac{3}{4} F(4 - 2\mu - 8\lambda - 8\lambda^2 \mu + 4\lambda^2 + 4\lambda^2 \mu + 5\lambda^2 - 4\lambda^2 \mu - 4\mu + 8\lambda \mu + 10 - 6 - 5\lambda^2) / (L w \mu (2\mu + 3)) - \frac{F(-2)(\lambda^2 - 2 + 2)(\mu + \lambda)}{L w \mu (2\mu + 3)^3}$$

The largest term is the third one, and it is independent of η (hence y), so we see that for h/L sufficiently small, the beam theory solution is a good approximation to the (unknown) elasticity solution. We integrate the leading term to get the deflection, and add in a constant to make the deflection at the origin zero

```
STUDENT > deflection:=L*int(op(3,"),xi)+Cv;
```

```
STUDENT > xi:=0:Cv:=solve(deflection,Cv):xi:='xi':  
deflection:=factor(deflection);
```

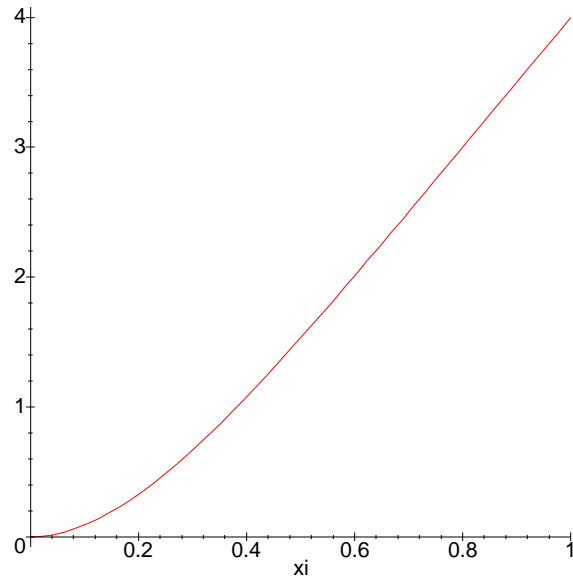
$$deflection := -\frac{1}{5} \frac{F(\mu + \lambda) \xi^2 (\xi^3 - 5\xi^2 + 10\xi - 10)}{w \mu (2\mu + 3\lambda) \epsilon^3}$$

The variation is in the numerator, so we can set up for a dimensionless plot

```
STUDENT > num:=numer(deflection);
```

```
STUDENT > factor("/F/(mu+lambda));
```

```
STUDENT > plotdef:="":  
plot(plotdef,xi=0..1);
```



the maximum deflection is at xi = 1 (x = L)

```
STUDENT > xi:=1:maxdef:=factor(deflection);xi:='xi':
```

$$maxdef := \frac{4}{5} \frac{F(\mu + \lambda)}{w \mu (2\mu + 3\lambda) \epsilon^3}$$

And that agrees with the result in Crandall & Dahl (solution to problem 8.14)